SPECIAL INSPECTION AGREEMENT

CROOK COUNTY COMMUNITY DEVELOPMENT BUILDING DEPARTMENT

300 NE 3RD ST, RM12, PRINEVILLE, OR 97754 541-447-3211



Project Address:	Permit #:	
Risk Category (1604.5):	Seismic Design Category (1613.3.5):	
PART 1 – STATEMENT OF SPECIAL INSPECT	ONS	
building permit cannot be issued until a statemen responsible charge and approved by the Building shall identify the materials, systems, components inspection; the type and extent of each test; addit	ted in accordance with Oregon Structural Specialty Code ("OSSC") chapter to f special inspections has been submitted by the registered design profess Safety Division. Pursuant to OSSC section 1704, the statement of special in and work requiring special inspection or testing; the type and extent of each conal requirements for seismic resistance special inspection or testing; and five it will be continuous special inspection or periodic special inspection.	ional in nspections n special
Check the items listed below for which special ins sections indicated in parentheses) and complete	pection is required to be performed as required by code (please refer to OS and attach the related Schedule as required for each item checked.	sc
 □ Shop fabrication of load-bearing members (1) □ Special inspection is not required where fabricator's certificate of compliance or in □ Steel construction (1705.2): attach Scheduling 	the work is done on the premises of an approved fabricator (1704.2.5,2): attac egistration by a nationally recognized accrediting authority as approved fabrica	ch approved ator
Concrete construction (1705,3): attach Sche		
Masonry construction (1705,4): attach Sche		
☐ Wood construction (1705.5): attach Schedus		
Soils (1705.6): attach Schedule E1	<i>60</i>	
Driven deep foundations (1705.7): attach Sc	t- dut- 50	
Cast in place deep foundations (1705.8): atta		
Helical pile foundations (1705.9): attach Sch		
Sprayed fire-resistant materials (1705.13): al		
Mastic and intumescent fire-resistant coating		
Exterior insulation and finish systems (1705.		
Special cases (1705.1.1): attach Schedule (;	
Smoke control systems (1705,17)		
For Risk Categories III or IV check main wind or sei.	smic resistance items listed below.	
Special inspections for seismic resistance (1) Testing and qualification for seismic resistance For structures assigned to Seismic Design Categoria Structural observations (1704.5): attach School	ce (1705.12): attach Schedule J es D, E or F, check the item below as applicable.	
Responsible Design Professional's Name (<i>Please</i>	Print) Responsible Design Professional's Signature	Date
PART 2 – ACKNOWLEDGEMENTS		
Owner, or responsible design professional acting a Festing Agencies and Structural Observer identifies as specified in the above Statement of Special Insor or a Certificate of Occupancy issued by the Buildir	as Owner's agent, hereby acknowledges that it shall employ the Testing Age d below who shall provide the special inspections, testing or structural obserpections during construction. Before a request for a final inspection can be g Safety Division, each Testing Agency or Structural Observer as identified vision documenting required special inspections and correction of any discre	rvations granted below
Owner Name (Please Print)	Owner's Signature	Date
General Contractor Name (Please Print)	Contractor's Signature	Date
esting Agency "A" Name (Please Print)	Testing Agency A's Signature	Date
esting Agency "B" Name (Please Print)	Testing Agency B's Signature	Date
esting Agency "C" Name (Please Print)	Testing Agency C's Signature	Date
Structural Observer's Name (Please Print)	Structural Observer's Signature	Date
	Building Safety Division Approval	Date

Project Address:	Permit #:	
Risk Category (1604.5):	Seismic Design Category (1613.3.5):	
SCHEDULE A - STEEL CONSTRUCTION (SEE OSS	C TABLE 1705.2 AND SECTION 1705.2 EXCEPTIONS)	
1. Periodic material verification of high-strength to		
Identification markings to conform to ASTM 6	standards specified in the approved construction documents.	
Manufacturer's certificate of compliance requ	lired.	
2. Inspection of high-strength bulting:		
Periodic inspection of snug tight joints.		
 Periodic inspection of slip-critical connections methods. 	s, turn-of-the-nut with match-making, direct-tension indicator or twist-off bolt	
Continuous inspection of slip-critical connect	ions, calibrated wrench or turn-of nut without match-making.	
 3. Periodic material verification of structural steel 	:	
Identification of markings to conform to AISC	360 specification in the approved construction documents.	
	form to ASTM standards specified in the approved construction documents.	
Manufacturer's certified test reports.		
4. Material verification of cold-formed steel deck:		
Manufacturer's certified test reports,		
5. Periodic material verification of weld filler material		
_	specification in the approved construction documents.	
Manufacturer's certificate of compliance requ	ired.	
6. Inspection of welding:		
a,) Structural steel and cold-formed steel deck:		
 Continuous inspection of complete and participant 	al penetration groove welds.	
 Continuous inspection of multi-pass fillet wel 	ds,	
Continuous inspection of single-pass fillet we		
Continuous inspection of plug and slot welds	i _e	
Periodic inspection of single-pass fillet welds	· ≤ 5/16",	
Periodic inspection of floor and deck welds,		
	lled with an automatically timed stud welding machine per AWS D1.1 Section 7.	
Periodic inspection of welded studs installed	with an automatically timed stud welding machine per AWS D1.1 Sections 7 & 7,8.1.	
b.) Inspection of reinforcing steel welding:		
Periodic verification of weldability of reinforci		
 Continuous inspection of reinforcing steel re- boundary elements of special concrete shear 	sisting flexural and axial forces in intermediate and special moment frames, r walls and shear reinforcement	
 Continuous inspection of shear reinforcement 	nt.	
Periodic inspection of other reinforcing steel.		
 7. Inspection of steel frame joint details for comp (Cold-formed steel trusses spanning 60 feet or gr 	liance with approved construction documents: reater.)	
Periodic details such as bracing and stiffenir	ig.	
Periodic member locations.		
Periodic application of joint details at each of	onnection.	
Notes:		

Project Agar	s: Permit #:
Risk Categor	(1604.5): Seismic Design Category (1613.3.5):
SCHEDULE	- CONCRETE CONSTRUCTION (SEE OSSC TABLE 1705.3 AND SECTION 1705.3 EXCEPTIONS)
1. Pe 2. Ins 3. Pei 4. Pei 5. Pei 6. Coi detern 7. Coi 8. Pei 9. Insi C 10. Pe	dic inspection of reinforcing steel, including prestressing tendons, and placement. ction of reinforcing steel welding in accordance with Schedule A, Item 6b. (attach Schedule A) dic inspection of anchors installed in concrete where allowable loads have been increased or where strength design is used. dic inspection of anchors post-installed in hardened concrete members. (See footnote "b" Table 1705.3) dic verification of required design mix. nuously, at time concrete is sampled to fabricate specimens for strength test, perform slump and air content tests, and e the temperature of the concrete. nuous inspection of concrete and shotcrete for proper application techniques. dic inspection for maintenance of specified curing temperature and techniques. ction of prestressed concrete: tinuous inspection of application of prestressing forces. tinuous inspection of grouting of bonded prestressing tendons in the seismic force-resisting system. addic inspection of erection of precast concrete members.
u 11. Pe	odic verification of in-situ concrete strength, prior to stressing of tendons in post-tensioned concrete and prior to removal of and forms from beams and structural slabs.
☐ 12. Pe	odic inspection of formwork for shape, location and dimensions of the concrete member being formed.
Notes:	

Project Address:	Permit #:
Risk Category (1604.5):	Seismic Design Category (1613,3,5):
1.19.2, OSSC 1705.4 AND 1705.4 EXCEPTIONS) Require designed masonry, glass unit masonry or masonry veneer Minimum tests required: 1.) Verification of Slump flow a with Specification Article 1.5 B.1.b.3 for self-consolidating a Article 1.4 B prior to construction, except where specifically	nd Visual Stability Index (VSI) as delivered to the project site in accordance prout. 2.) Verification of Fm and flags in accordance with Specification exempted by this Code.
Periodically verify compliance with the approved su	bmittals
2. Verification as masonry construction begins:	
Periodic verification of proportions of site-prepare	
Periodic inspection of construction of mortar joints	
Periodic inspection of placement of reinforcement Periodic inspection of prestressing technique.	t, connectors, and prestressing tendons and anchorages.
Periodic inspection of prestressing technique. Periodic verification of grade and size of prestress	sing tendons and anchorages
	C masonry for the first 5000 square feet of AAC masonry and periodic after the
☐ 3. Verification prior to grouting:	
Periodic inspection of grout space prior to grouting	
	inforcement and anchor bolts, and prestressing tendons and anchorages.
•	nt, connectors, and prestressing tendons and anchorages.
	d grout and prestressing grout for bonded tendons,
Periodic verification of construction of mortar joint	S,
4. Inspections during masonry construction:	watered alexander
 Periodic verification of the size and location of str Periodic verification of the type, size, and location members, frames, or other construction. 	uctural elements, of anchorage to masonry to structural
Continuous inspection of welding of reinforcing ba	ars.
	and protection of masonry during cold weather (<40 deg F) or hot weather (>90
Continuous inspection of the application and mea	surement of prestressing force.
Continuous inspection of the placement of grout a	and prestressing grout for bonded tendons is in compliance.
square feet of AAC masonry and periodic for afte	
5. Periodic observation of preparation of grout speci	mens, mortar specimens and/or prisms.
1.19.3, OSSC 1705.4 AND 1705.4 EXCEPTIONS) Require Minimum tests required: 1.) Verification of firm and f'aac iduring construction. 2.) Verification of proportions of materials and the state of the st	in accordance with Article 1,4 B prior to construction and for every 5000 sq. ft. rials in premixed or preblended mortar, prestressing rout, and grout other e. 3.) Verification of Slump flow and Visual Stability Index (VSI) as delivered .5 B.1.b.3 for self-consolidating grout.
2. Verification that the following are in compliance:	su submittais
	nortar, grout and prestressing grout for bonded tendons.
Periodic inspection of placement of masonry unit	
3	nent, connectors and prestressing tendons and anchorages.
Continuous inspection of grout space prior to gro	uting
Continuous inspection of placement of grout and	prestressing rout for bonded tendons.
Periodic verification of size and location of structu	
Periodic verification of type, size, and location of frames, or other construction.	anchors including other details of anchorage of masonry to structural members,
Continuous inspection of welding of reinforcing back	
deg F).	and protection of masonry during cold weather (<40 deg F) or hot weather (>90
Continuous inspection of application and measur	
	sonry units and construction of thin-bed mortar joints.
Continuous verification of the properties of thin-be	
3. Continuous observation of preparation of grout s	эвоннено, тока эрвоннено огрната.
Notes:	

Project Address:Risk Category (1604.5):		Permit #:	
		Seismic Design Category (1613.3.5):	
SCHEDU	LE D - WOOD CONSTRUCTION (OSSC 1705.5)		
		nts and assemblies shall be in accordance with OSSC 1704.2.5. approved as an approved fabricator per OSSC 1704.2.5 and 1704.2.5.2	
	For high-load diaphragms designed in accordance v thickness, nominal size of framing members at adjoi and spacing between fasteners and at edge margin:	with OSSC 2306.3.2, periodic verification of sheathing panel grade and ning panel edges, fastener diameter and length, the number of fastener lines s with approved building plans.	
	Metal-plate-connected wood trusses spanning 60 fe restraint/bracing and the permanent individual truss submittal package.	et or greater shall provide verification that the temporary installation member restraint/bracing are installed in accordance with approved truss	
Notes:			

Proje	ct Address:	Permit #:
Risk	Category (1604.5):	Seismic Design Category (1613.3.5):
SCHE	EDULE E1 – SOILS (SEE OSS	C TABLE 1705.6)
	Periodic verification that excave Periodic classification and test Continuous verification of use	rials below shallow foundations are adequate to achieve the design bearing capacity. rations have extended to proper depth and have reached proper material. ing of compacted fill materials. of proper materials, densities and lift thicknesses during placement and compaction of compacted fill, ed fill, periodically observe subgrade and verify that the site has been properly prepared.
Notes	<u> </u>	
SCHE	DULE E2 - DRIVEN DEEP F	OUNDATIONS (SEE OSSC TABLE 1705.7)
	Continuous determination of car Continuous observation of eler Continuous verification of place penetration, determine require foundation element. For steel elements, perform ad For concrete elements and con Schedule B).	ement materials, sizes and lengths comply with the requirements of approved construction documents, apacities of test elements and conduct additional load tests, as required. ment driving operations and maintain complete and accurate records for each element, ament locations and plumbness, confirm type and size of hammer, record number of blows per foot of dipenetrations to achieve design capacity, record tip and butt elevations and document any damage to additional special inspections in accordance with OSSC 1705.2. (attach Schedule A). Increte-filled elements perform additional special inspections in accordance with OSSC 1705.3. (attach in additional inspections as determined by the registered designed professional in responsible charge.
Notes	·	
SCHE	DULE E3 - CAST-IN-PLACE	DEEP FOUNDATIONS ELEMENTS (SEE OSSC TABLE 1705.8)
	Continuous verification of place embedment into bedrock (if ap. For concrete elements perform For masonry piers perform add	ng operations and maintain complete and accurate records for each element. ement locations and plumbness; and confirm element diameters, bell diameters (if applicable), lengths, plicable) and adequate end bearing capacity. Record concrete or grout volumes. additional special inspections in accordance with OSSC 1705.3 (attach Schedule B). itional special inspections in accordance with OSSC 1705.4 (attach Schedule C1 or C2).
SCHE	DULE E4- HELICAL PILE FO	UNDATIONS (SEE OSSC TABLE 1705.9)
	final depth, final installation torc	ne installation of helical piles. Record the installation equipment used, pile dimensions, tip elevations, qued and other pertinent installation data as required by the registered design professional in red geotechnical report and approved construction documents prepared by a registered design etermine compliance.
lotes:		

Risk Category (1604.5):	Project	Ado	dress:	Permit #:
The prepared surface of structural members to be sprayed shall be inspected before the application of the SFRM. Vorification in accordance with the manufacturer's written instructions of ambient temperature before and after application, substrate conditions, and protection provided. Determination of the thickness of SFRM in accordance with ASTM € 605 as required by the approved fire-resistant design. □ Floor, roof, and wall assemblies shall have sampling for determining the thickness of SFRM shall be determined in accordance with ASTM € 605 making not less than 4 measurements per 1,000 sq ft of the sprayed area of each floor or part thereof in each story. □ Cellular deck shall have sampling of the SFRM thickness in a 12 inch by 12 inch area. A minimum of 4 measurements shall be made, located symmetrically within the square area. □ Fluted decks shall have sampling of the SFRM thickness in a selected square area of 12 inches by 12 inches. A minimum of 4 measurements shall be made, located symmetrically within the square are, including one each of the following: valley, crest and sides. The average shall be reported. □ Structural members shall have sampling for determining the thickness of SFRM shall be in accordance with ASTM E 605 and not less than 25 percent of the structural members on each floor. □ Beam and girder samplings of SFRM thickness shall be made at 9 locations around the beam or girder at each end of a 12 inch length. □ Vide-flanged column sampling of SFRM thickness at 12 locations around the column at each end of a 12 inch length. □ Hollow structural section and pipe column sampling of SFRM thickness shall be a minimum of 4 locations around the column at each end of a 12 inch length. □ Floor, roof, and wall assembly density sampling shall be each floor at the rate of not less than one sample for every 2500 square feet of floor are or portion thereof in each story. □ Beams, girders, trusses, and columns density sampling shall be ach floor, roof, and wall assembly at a rate of not less th	Risk C	ateg	ory (1604.5):	Seismic Design Category (1613.3.5):
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Determination of the thickness of SFRM in accordance with ASTM E 605 as required by the approved fire-resistant design. Floor, roof, and wall assemblies shall have sampling for determining the thickness of SFRM shall be determined in accordance with ASTM E 605 making not less than 4 measurements per 1,000 sq ft of the sprayed area of each floor or part thereof in each story. Cellular deck shall have sampling of the SFRM thickness in a 12 inch by 12 inch area. A minimum of 4 measurements shall be made, located symmetrically within the square area. Fluted decks shall have sampling of the SFRM thickness in a selected square area of 12 inches by 12 inches. A minimum of 4 measurements shall be made, located symmetrically within the square are, including one each of the following: valley, crest and sides. The average shall be reported. Structural members shall have sampling for determining the thickness of SFRM shall be in accordance with ASTM E 605 and not less than 25 percent of the structural members on each floor. Beam and girder samplings of SFRM thickness shall be made at 9 locations around the beam or girder at each end of a 12 inch length. Joist and truss sampling shall be made at seven locations around the joist or truss at each end of a 12 inch length. Wide-flanged column sampling of SFRM thickness at 12 locations around the column at each end of a 12 inch length. Hollow structural section and pipe column sampling of SFRM thickness shall be a minimum of 4 locations around the column at each end of a 12 inch length. Determination of the density of SFRM in accordance with ASTM E 605 as required by the approved fire-resistant design. Floor, roof, and wall assembly density sampling shall be each floor at the rate of not less than one sample for every 2500 square feet or portion thereof of the sprayed area in each story. Beams, girders, trusses, and columns density sampling shall be each floor at the rate of not less than one sample for each 2,500 square feet of sprayed area, or		Vori	fication in accordance with the	manufacturer's written instructions of ambient temperature before and after application, substrate
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 Determination of the bond strength in accordance with ASTM E 736 of cured SFRM applied to floor, roof, wall assemblies, and structural members shall not be less than 150 psf. Bond strength sampling for floor, roof or wall assemblies for SFRM shall be each floor, roof, and wall assembly at a rate of not less than one sample for every 2,500 square feet of sprayed area, or portion thereof, in each story. Bond strength sampling for SFRM shall be selected from beams, girders, trusses, columns, and other structural framing members at the rate of not less than one sample for type of structural member for each 2,500 square feet of floor area or portion thereof in each story. Bond tests for primer, paint, and encapsulant bond tests shall be conducted when the SFRM is applied to a primed, painted or encapsulated surface for which acceptable bond-strength performance between these coatings and the SFRM has not been determined. 			ermination of the density of SF Floor, roof, and wall assem square feet or portion there Beams, girders, trusses, and	RM in aecordance with ASTM E 605 as required by the approved fire-resistant design. ably density sampling shall be each floor at the rate of not less than one sample for every 2500 to fit the sprayed area in each story. and columns density sampling shall be at a rate of not less than one sample for each type of
Bond strength sampling for SFRM shall be selected from beams, girders, trusses, columns, and other structural framing members at the rate of not less than one sample for type of structural member for each 2,500 square feet of floor area or portion thereof in each story. Bond tests for primer, paint, and encapsulant bond tests shall be conducted when the SFRM is applied to a primed, painted or encapsulated surface for which acceptable bond-strength performance between these coatings and the SFRM has not been determined.		stru	ermination of the bond strengt ctural members shall not be le Bond strenath sampling for fle	h in accordance with ASTM E 736 of cured SFRM applied to floor, roof, wall assemblies, and assembles than 150 psf. poor, roof or wall assemblies for SFRM shall be each floor, roof, and wall assembly at a rate of not
encapsulated surface for which acceptable bond-strength performance between these coatings and the SFRM has not been determined.			Bond strength sampling for S members at the rate of not le	FRM shall be selected from beams, girders, trusses, columns, and other structural framing
Notes:			encapsulated surface for whi	and encapsulant bond tests shall be conducted when the SFRM is applied to a primed, painted or ch acceptable bond-strength performance between these coatings and the SFRM has not been
	Notes:			

Project Address:Risk Category (1604.5):		Permit #:	
		Seismic Design Category (1613.3.5):	
Red app inst	plications of materials described in O	systems that are alternatives to materials and systems prescribed by OSSC, unusual design SSC, or materials and systems required to be installed in accordance with manufacturer's not contained in OSSC or referenced standards. For each item checked below, check the	
30000000000	Powder driven shot-in anchors: O Continuous O Periodic Underpinning: O Continuous O Continu	oriodic ng wall systems: Continuous Periodic Continuous Periodic t coatings (OSSC 1705.14). (OSSC 1705.16, High Rise buildings or buildings assigned a Risk Category III or IV).	
Notes:			

Project Address:		Permit #:
Risk Cate	gory (1604.5):	Seismic Design Category (1613.3.5):
SCHEDUL	E H - CONTRACTOR'S	STATEMENT OF RESPONSIBILITY FOR SEISMIC RESISTANCE OSSC 1704.4
con con res con stat as s	istruction of the main wind inponent listed in the state ponsibility to the Buildin itractor's statement of resi tement of special inspection specified by the registered nedule K:	portsibility shall containing the following for Contractor and each Subcontractor responsible for the dor seismic-force-resisting system, designated seismic systems or a wind or seismic-resisting ment of special inspections. The contractor and subcontractors shall submit a written statement of g Official and the owner prior to the commencement of work on the system or component. The ponsibility shall contain acknowledgement of the awareness of the special requirements contained in the on. Including: special inspections, testing or structural observations for seismic resistance are required it design professional on this Statement of Special Inspection and attached Schedule I, Schedule J or vareness of the special inspection requirements contained in the Statement of Special Inspections and
	Acknowledgement that of Safety Division.	control will be exercised to obtain conformance with the construction documents approved by the Building
	Procedures for exercising the reports.	g control within Contractor's organization, the method and frequency of reporting and the distribution of
	Identification and qualific	cations of the person(s) exercising such control and their position(s) in the organization,
Notes:		

Project Address:Permit #:		Permit #:	
Risk	Cate	egory (1604,5):	Seismic Design Category (1613.3.5):
SCH III an	EDU nd IV	ILE I -SPECIAL INSPECTIONS FOR SEISMIC R unless specifically exempt by the exceptions of 1	ESISTANCE (SEE OSSC SECTION 1705.11) Structures in Risk Category 704.2.
	Sei	sismic-force-resisting systems in structures assigned	to Seismic Design Categories C, D, E or F:
		Inspections for structural steel in accordance with Tasks), OSSC 1705.2 and Table 1705 (attach Sc	the quality assurance requirements of AISC 341 CH. J section J5 (Inspection hedule A). 1705.11.1
		1705.11.2	d during field gluing operations of element of the seismic force-resisting system.
		Periodic inspection of structural wood required for force-resisting system with fastener spacing 4 incl struts, braces, shear panels and hold-downs. 170	nailing, bolting, anchoring and other fastening of components with the seismic nes on center or less, including wood shear walls, wood diaphragms, drag 15.11,2
		Periodic inspection is required for cold-formed ste force-resisting system, 1705,11,3	el light-frame construction during welding operations of elements of the seismic
		fastening of components within the seismic for-res	light-framed construction for screw attachment, bolting, anchoring, and other isting system, including shear walls, braces, diaphragms, collectors (drag g is not gypsum board or fiberboard and is wood or steel sheets with fastener 1.11.3
	req per	quirements for certification by analysis, testing or exp	ations per OSSC 1705.12.3. Construction documents shall specify the perience data for nonstructural components and designated seismic systems quired by OSSC 1705.12 in structures assigned to Seismic Design Categories
		Periodic verification of labeling, anchorage or mou Schedule G).	nting systems conform to the certificate of compliance (for anchors attach
	Arc		in height and assigned to Seismic Design Categories D, E or F (1705,11,5):
		15 psf or veneer weighing more than 5 psf (for and	
		Periodic inspection during the anchorage of acces	
	Ц	Periodic inspection of the installation and anchorage	ge of suspended ceiling systems (for anchors attach Schedule G).
			signed to Seismic Design Categories C, D, E or F (1705,11.6):
		Periodic inspection during anchorage of electrical of Schedule G),	equipment for emergency or standby power systems (for anchors attach
	_	Periodic inspection is required during the anchorage Category E or F (for anchors attach Schedule G).	ge of other electrical equipment in structures assigned to Seismic Design
		Periodic inspection during the installation and the a mechanical units (for anchors attach Schedule G)	anchorage of piping systems carrying hazardous materials and their associated .
		Periodic inspection during the installation and the a anchors attach Schedule G).	anchorage of HVAC ductwork that will contain hazardous materials (for
		Periodic inspection during the installation and ancha nominal clearance of 0.25" or less between the e	norage of vibration isolation systems where the construction documents require equipment support frame and restraint (for anchors attach Schedule G).
	Stor	rage racks in structures assigned to Seismic Design	Category D, E, or F (1705.11.7):
			or greater in height (for anchors attach Schedule G).
	Peri		seismic isolator units and energy dissipation devices that are part of the
Notes:	-		

Project Address:	Permit #:	
Risk Category (1604.5):	Seismic Design Category (1613.3.5):	
SCHEDULE J -TESTING AND QUALIF	FICATION FOR SEISMIC RESISTANCE	
f esting and qualification specified of 1704.2 for seismic-force-resisting	In OSSC 1705.12,1 through 1705,12,4, unless exempt from special inspections by the exceptions in structures assigned to Seismic Design Categories C, D, E or F:	
earthouake induced flexural a	It reports for each shipment of reinforcing steel complying with ASTM A 615 used to resist and axial forces in special moment frames, special structural walls and coupling beams connecting actures assigned to Seismic Design Category B, C, D, E, or F, the reinforcement shall comply with	
Chemical tests in accordance	with ACI 318 section 3.5.2 to determine weldability of ASTM A 615 reinforcing steel	
Testing for structural steel as structures in Seismic Design	required by AISC 341 quality assurance OSSC 1705,1.2. See exception in OSSC 1705,12.2 for Category C,	
shrinkage strains in base met	nuities behind and adjacent to welds after joint completion where subject to through-thickness weld all thicker than 1.5". Acceptance criteria for nondestructive testing shall be as required in ASTM A criteria) as specified by the registered design professional on the construction documents.	
Designated seismic system in stru requirements of ASCE 7 section 1	ctures assigned to Seismic Design Category C, D, E, or F and subject to the certification 3.2.2 and comply with OSSC 1705.12.3.	
Active mechanical and electric certified exclusively on the ba accordance with ASCE 7 sec	cal equipment that must remain operable following the design earthquake ground motion shall be sis of approved shake table testing in accordance with ASCE 7 section 13,2.5 or data in tion 13.2.6.	
shall be certified by the manu	substances and assigned an importance factor >1.5 in accordance with ASCE 7 section 13.1.3 facturer as maintaining containment following the design earthquake ground motion by analysis, in accordance with ASCE 7 section 13.2.5, or data in accordance with ASCE 7 section 13.2.6.	
Architectural, mechanical and electrogramments of ASCE 7 section 2	ctrical components in structures assigned Seismic Design Categories C, D, E or F and where the 13.2.1 item 2 are met by submittal of manufacture's certification and comply with OSSC 1705.12.3:	
registered design professiona	at the component is seismically qualified by one or more of the following and as specified by the ill on the construction documents:	
Analysis.		
	ith the alternative set forth in ASCE 7 section 13.2.5.	
	dynamic characteristics and forces.	
	rdance with the alternative set forth in ASCE 7 section 13.2.6.	
Testing of seismic isolation system	n components in accordance with OSSC 1705-12.4 and ASCE 7 section 17.8.	
Notes:		

Proje	ct Address:	Permit #:
Risk	Category (1604.5):	Seismic Design Category (1613.3,5):
SCH	EDULE K - STRUCTURAL OB	SERVATIONS OSSC 1704.5
the from	equency and extent of structura it to the Building Official a <i>writt</i> e	vations, the structural observer shall submit to the Building Official a <i>written statemen</i> t identifying il observations. At the conclusion of the work included in the permit, the structural observer shall <i>en statement</i> that the site visits have been made and identify any reported deficiencies which, to knowledge, have not been resolved.
		ic resistance: Structural observation shall be provided for structures assigned to Seismic Design are of the following conditions exist. OSSC 1704.5.1
	Structural observations for structure is assigned to Se and is greater than 2 stories ab Structural observations for structural observations f	ctures classified as Risk Categories III or IV per OSSC TABLE 1604.5, ctures with height greater than 75 ft above the base. ismic Design Category E, is classified as Risk Category I or II in accordance with OSSC Table 1604.5, ove grade plane. ctures when so designated by the registered design professional in responsible charge of the design. ctures when specifically required by the Building Official.
Struc accord	dance OSSC 1609.3.1 exceeds Structural observations for struc Structural observations for struc Structural observations for struc	quirements: Structural observation shall be provided for structures sited where Vasa as determined in 110 mph, where one or more of the following conditions exist. OSSC 1704.5.2 stures classified as Risk Categories III or IV per OSSC TABLE 1604.5. stures with height greater than 75 ft above the base. stures when so designated by the registered design professional in responsible charge of the design. stures when specifically required by the Building Official.
Notes	:	